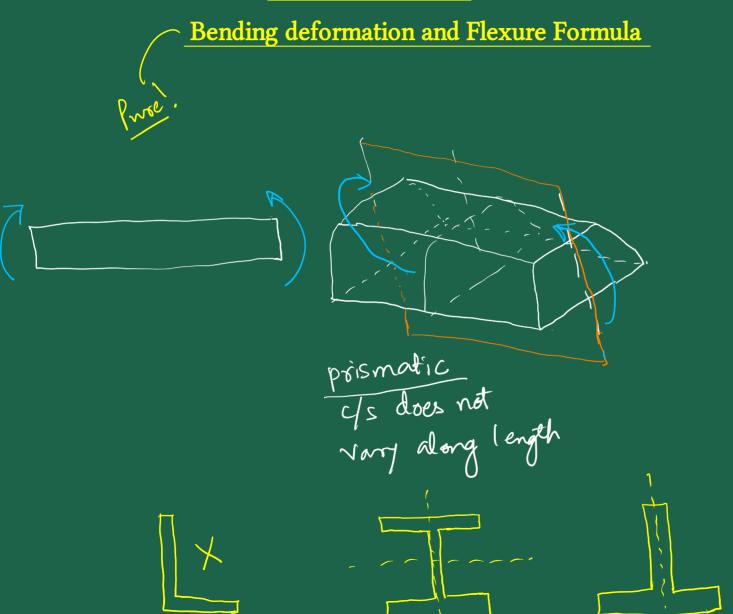
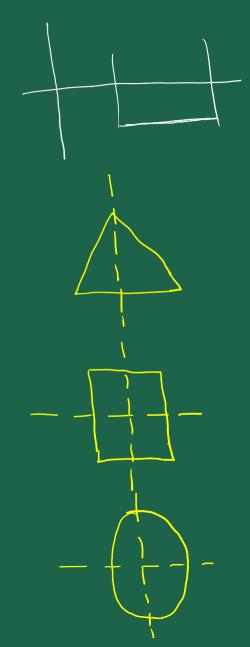
## Bending of Beams





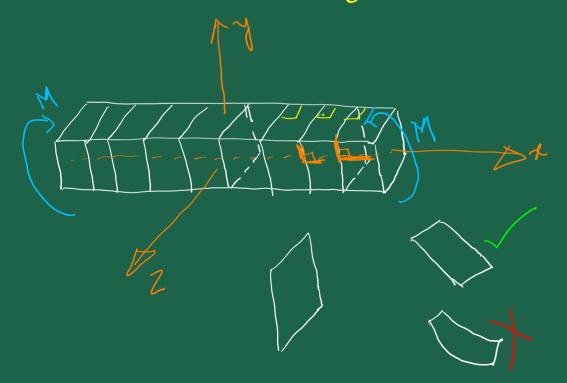
$$M = -\int y \, dF$$

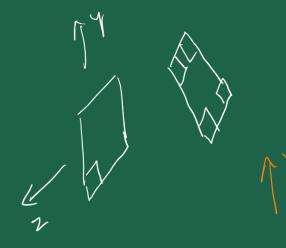
$$M = -\int y \, \sigma_{xx} \, dA$$

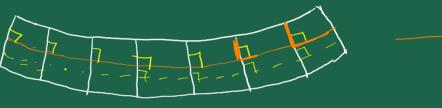
$$O = \int \sigma_{xx} \, dA$$

## Enler - Bernoulli

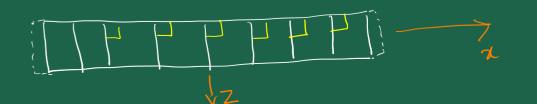
- # Plane sections remain plane
- # Plane sections remain perpendicular to the longitudinal axis
- # Plane sections do not change dimensions







$$\mathcal{E}_{yy}=0$$
,  $\mathcal{E}_{zz}=0$ ,  $\mathcal{E}_{nx}\neq0$   
 $\mathcal{E}_{zz}=0$ ,  $\mathcal{E}_{nx}\neq0$   
 $\mathcal{E}_{zz}=0$ ,  $\mathcal{E}_{nx}\neq0$ 

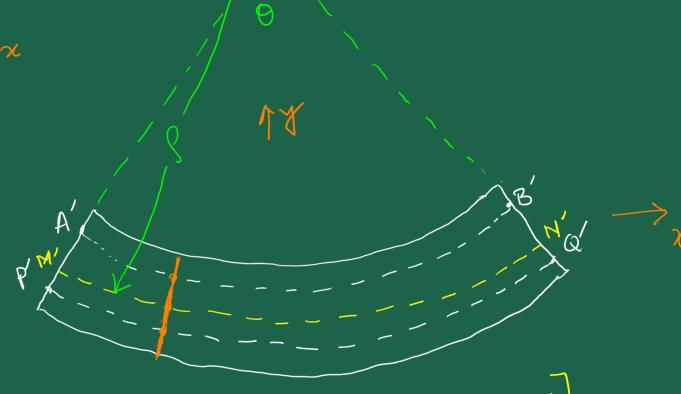


$$\begin{aligned}
\delta_{xy} &= \delta_{yz} = \delta_{zx} = 0 \Rightarrow \tau_{xy} = \tau_{yz} = \tau_{zx} = 0 \\
\varepsilon_{xx} &= \frac{1}{E} \left[ \sigma_{xx} - \delta(\sigma_{yy} + \sigma_{zz}) \right] &\rightarrow \sigma_{xx} = \int_{I} \left( \varepsilon_{xx}, \varepsilon_{yy}, \varepsilon_{yz} \right) \\
\varepsilon_{yy} &= \frac{1}{E} \left[ \sigma_{yy} - \delta(\sigma_{xx} + \sigma_{zz}) \right] &\rightarrow \sigma_{xy} = \int_{I} \left( \varepsilon_{xx}, \varepsilon_{yy}, \varepsilon_{yz} \right) \\
\varepsilon_{zz} &= \frac{1}{E} \left[ \sigma_{zz} - \delta(\sigma_{xx} + \sigma_{yy}) \right] &\leftarrow \tau_{zz} = \int_{I} \left( \varepsilon_{xx}, \varepsilon_{yy}, \varepsilon_{yz} \right) \\
\varepsilon_{zz} &= \int_{I} \left( \varepsilon_{xx} + \sigma_{yy} \right) &\leftarrow \tau_{zz} = \int_{I} \left( \varepsilon_{xx}, \varepsilon_{yy}, \varepsilon_{yz} \right) \\
\varepsilon_{zz} &= \int_{I} \left( \varepsilon_{xx} + \sigma_{yy} \right) &\leftarrow \tau_{zz} = \int_{I} \left( \varepsilon_{xx}, \varepsilon_{yy}, \varepsilon_{yz} \right) \\
\varepsilon_{zz} &= \int_{I} \left( \varepsilon_{xx} + \sigma_{yy} \right) &\leftarrow \varepsilon_{zz} = \int_{I} \left( \varepsilon_{xx}, \varepsilon_{yy}, \varepsilon_{yz} \right) \\
\varepsilon_{zz} &= \int_{I} \left( \varepsilon_{xx} + \varepsilon_{yy} \right) &\leftarrow \varepsilon_{zz} = \int_{I} \left( \varepsilon_{xx}, \varepsilon_{yy}, \varepsilon_{yz} \right) \\
\varepsilon_{zz} &= \int_{I} \left( \varepsilon_{xx} + \varepsilon_{yy} \right) &\leftarrow \varepsilon_{zz} = \int_{I} \left( \varepsilon_{xx}, \varepsilon_{yy}, \varepsilon_{yz} \right) \\
\varepsilon_{zz} &= \int_{I} \left( \varepsilon_{xx} + \varepsilon_{yy} \right) &\leftarrow \varepsilon_{zz} = \int_{I} \left( \varepsilon_{xx}, \varepsilon_{yy}, \varepsilon_{yz} \right) \\
\varepsilon_{zz} &= \int_{I} \left( \varepsilon_{xx} + \varepsilon_{yy} \right) &\leftarrow \varepsilon_{zz} = \int_{I} \left( \varepsilon_{xx}, \varepsilon_{yy}, \varepsilon_{yz} \right) \\
\varepsilon_{zz} &= \int_{I} \left( \varepsilon_{xx} + \varepsilon_{yy} \right) &\leftarrow \varepsilon_{zz} = \int_{I} \left( \varepsilon_{xx}, \varepsilon_{yy}, \varepsilon_{yz} \right) \\
\varepsilon_{zz} &= \int_{I} \left( \varepsilon_{xx} + \varepsilon_{yy} \right) &\leftarrow \varepsilon_{zz} = \int_{I} \left( \varepsilon_{xx}, \varepsilon_{yy}, \varepsilon_{yz} \right) \\
\varepsilon_{zz} &= \int_{I} \left( \varepsilon_{xx} + \varepsilon_{yy} \right) &\leftarrow \varepsilon_{zz} = \int_{I} \left( \varepsilon_{xx}, \varepsilon_{yy}, \varepsilon_{yz} \right) \\
\varepsilon_{zz} &= \int_{I} \left( \varepsilon_{xx} + \varepsilon_{yy} \right) &\leftarrow \varepsilon_{zz} = \int_{I} \left( \varepsilon_{xx}, \varepsilon_{yy}, \varepsilon_{yz} \right) \\
\varepsilon_{zz} &= \int_{I} \left( \varepsilon_{xx} + \varepsilon_{yy} \right) &\leftarrow \varepsilon_{zz} = \int_{I} \left( \varepsilon_{xx} + \varepsilon_{yy} \right) \\
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\varepsilon_{zz} &= \int_{I} \left( \varepsilon_{xx} + \varepsilon_{yy} \right) &\leftarrow \varepsilon_{zz} = \int_{I} \left( \varepsilon_{xx} + \varepsilon_{yy} \right) \\
\varepsilon_{zz} &= \int_{I} \left( \varepsilon_{xx} + \varepsilon_{xy} \right) &\leftarrow \varepsilon_{xy} + \varepsilon_{xy$$

$$\varepsilon_{xx} \neq 0$$
;  $\sigma_{xx} \neq 0$ , Rest avre  $0$ 



-> Neutral Surface



$$\varepsilon_{xx} = \frac{A'B' - AB}{AB}$$
  $\left[AB = MN = M'N'\right]$ 

$$=\frac{(\beta-\gamma)\theta-\beta\theta}{\beta}=\frac{-\gamma}{\beta}$$

$$\mathcal{E}_{\chi\chi} = -\frac{4}{S} \Rightarrow \mathcal{T}_{\chi\chi} = \mathcal{E}_{\chi\chi} = -\mathcal{E}_{\chi}$$

Now, 
$$M = -\int_{A} y \int_{XX} dA$$

$$=-\int_{A}y\left(-E\frac{y}{s}\right)dA$$

$$= \int_{A}^{E} \frac{E}{S} y^{2} dA$$

$$\frac{1}{2} = \frac{1}{2} \int_{A} y dA \qquad \frac{1}{2} = \frac{1}{2} \int_{A} y dA \qquad \frac{1}{2} = \frac{1}{2} \int_{A} y dA$$

(ydA First woment of area

Area moment of inertia

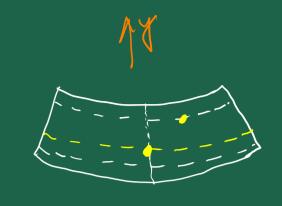
OR

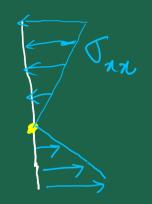
Second moment of area)

$$\mathcal{T}_{\chi\chi} = -E\frac{y}{s} + \left[ s = \frac{EI}{M} \right]$$

$$\int_{aa} = -M \frac{y}{I}
 \rightarrow Bending stress$$

7>0 > Cm <0 For points lying above the neutral surface: : y<0 > Tro > 0





T L L y dA = y A  $O = \int_{Ax} dA$ Nentral  $\Rightarrow 0 = \int \left(-\frac{My}{I}\right) dA$ > x 7 JydA = 0 7 x-anis passes through
the centroid But we had chosen our

x-anis to lie along the neutral surface

The implication is that the neutral surface passes through the centroid.

$$T_{xx} = -\frac{My}{T}$$
  $\rightarrow$   $T_{max} = -\frac{Mc}{T}$  [c: entremities of y]
$$= -\frac{M}{T/c}$$

$$= -\frac{M}{(S)} \rightarrow \text{Section Modulus}$$

1. A box beam is constructed from four pieces of wood, glued together as shown. If the moment acting on the cross section is 10 kN·m, determine the stress at points A and B.  $[\sigma_A = -6.21 \text{ MPa}, \sigma_B = 5.17 \text{ MPa}]$ 

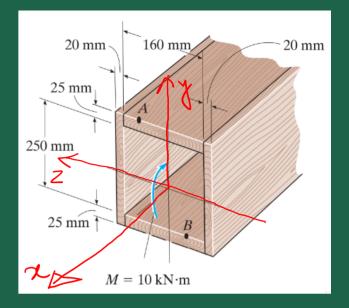
$$T = T_1 - T_2$$

$$T_1 = -\frac{M_Y}{T}$$

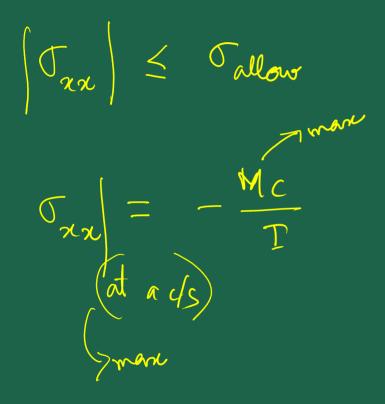
Rectangular cls

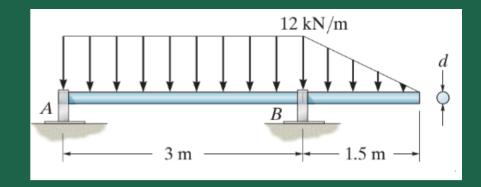
T = 1 bh3

12

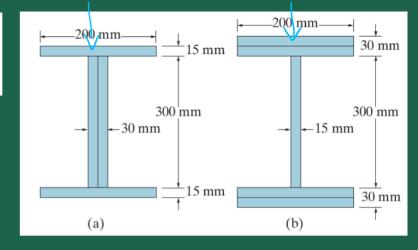


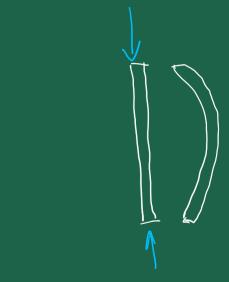
2. The shaft is supported by smooth journal bearings at A and B that only exert vertical reactions on the shaft. Determine its smallest diameter d if the allowable bending stress is  $\sigma_{\text{allow}} = 180 \text{ MPa}$ .



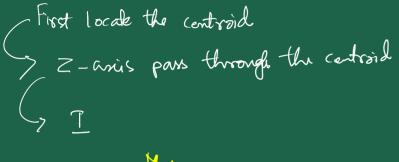


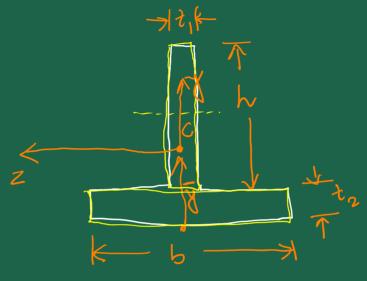
3. Two designs for a beam are to be considered. Determine which one will suport a moment of  $M = 150 \text{ kN} \cdot \text{m}$  with the least amount of bending stress. What is that stress? [Design (b);  $\sigma_{\min} = 74.7 \text{ MPa}$ ]

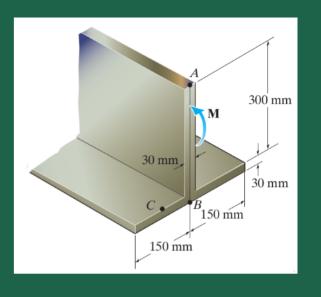




5. If the beam is subjected to an internal moment of M = 100 kN·m, determine the bending stress developed at points A, B, and C.  $[\sigma_A = 122 \text{ MPa (C)}, \sigma_B = 51.1 \text{ MPa (T)}, \sigma_C = 35.4 \text{ MPa (T)}]$ 





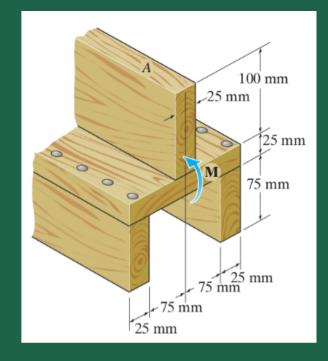


Centroid (c)
$$ht_1\left(t_2+\frac{h}{2}\right) + bt_2\frac{t_2}{2} = y\left(ht_1+bt_2\right) \rightarrow y$$

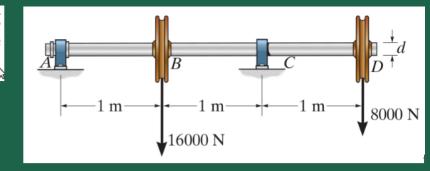
$$Second moment of wrea (T):$$

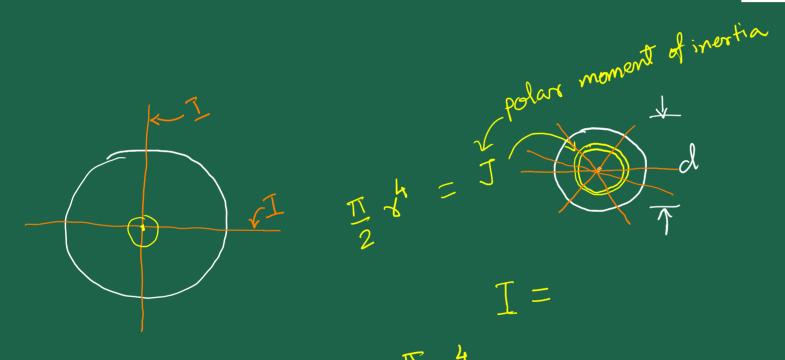
$$T = \frac{1}{12}t_1h^3 + ht_1\left(t_2+\frac{h}{2}-y\right) + \frac{1}{12}bt_2^3 + \left(y-\frac{t_2}{2}\right)bt_2$$

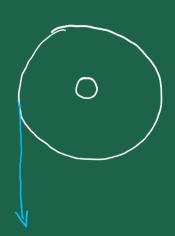
4. If the beam is subjected to an internal moment of M=3 kN·m, determine the maximum tensile and compressive stress in the beam. Also, sketch the bending stress distribution on the cross section.  $[\sigma_{\text{max,c}} = 13 \text{ MPa}, \sigma_{\text{max,t}} = 9.5 \text{ MPa}]$ 



6. The shaft is supported by a smooth thrust bearing at A and a smooth journal bearing at C. If the material has an allowable bending stress of  $\sigma_{\text{allow}} = 168 \text{ MPa}$ , determine the required minimum diameter d of the shaft to the nearest mm. [75 mm]



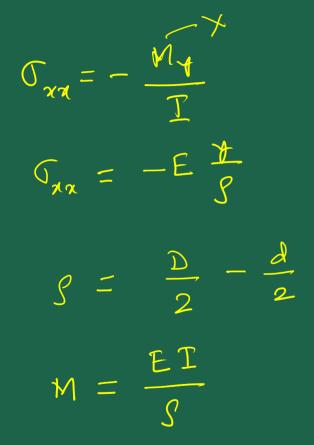


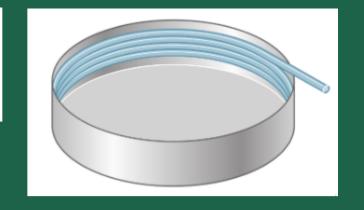


$$T + T = J \Rightarrow 2T = J = \frac{\pi}{2} x^4$$

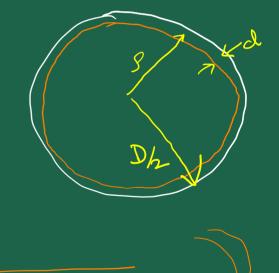
$$\therefore T = \frac{\pi}{4} x^4$$

8. Straight rods of 6 mm diameter and 30 m length are stored by coiling the rods inside a drum of 1.25 m inside diameter. Assuming that the yield strength is not exceeded, determine the maximum stress in a coiled rod and the corresponding bending moment in the rod. Use E = 200 GPa. [965 MPa, 20.5 N·m]





$$\frac{1}{s} = \frac{M}{E^{s}}$$



9. A copper strip and an aluminium strip are bonded together to form the composite beam shown. Knowing that the beam is bent about a horizontal axis by a couple of moment  $M = 35 \text{ N} \cdot \text{m}$ , determine the maximum stress in the aluminium strip and the copper strip. The Young's modulus of copper is 105 GPa and that of aluminium is 75 GPa.

[-56.0 MPa, 68.4 MPa]

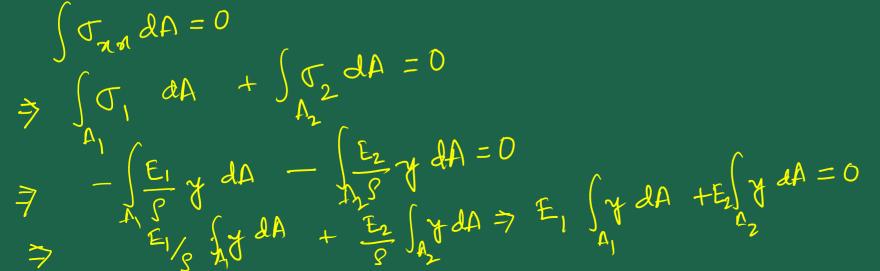
$$\int \int \chi dA = 0$$

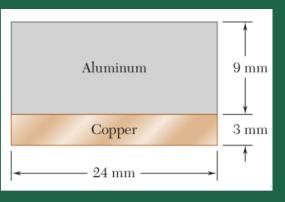
$$\mathcal{E}_{\chi\chi} = -\frac{Y}{S} \text{ (Composite)}$$

$$\int \chi \int_{\chi\chi} dA = -M$$

$$\int_{2} = E_{1} \mathcal{E}_{\chi\chi} = -E_{1} \frac{Y}{S}$$

$$\int_{2} = E_{2} \mathcal{E}_{\chi\chi} = -E_{2} \frac{Y}{S}$$





$$\int_{\gamma} \nabla_{z_{N}} dA = -M$$

$$\Rightarrow \int_{A_{1}} \nabla \nabla_{z_{N}} dA + \int_{A_{2}} \nabla \nabla_{z_{N}} dA = -M$$

$$\Rightarrow -\int_{A_{1}} \nabla_{z_{N}} \nabla A - \int_{A_{2}} \nabla_{z_{N}} \nabla A = -M$$

$$\Rightarrow \frac{E_{1}}{S} \int_{A_{1}} \nabla A + \frac{E_{2}}{S} \int_{A_{2}} \nabla A = M$$

$$\Rightarrow \frac{E_{1}}{S} \int_{A_{1}} \nabla A + \frac{E_{2}}{S} \int_{A_{2}} \nabla A = M$$

$$\Rightarrow \frac{E_{1}}{S} \int_{A_{1}} \nabla A + \frac{E_{2}}{S} \int_{A_{2}} \nabla A = M$$

$$\Rightarrow \frac{E_{1}}{S} \int_{A_{1}} \nabla A + \frac{E_{2}}{S} \int_{A_{2}} \nabla A = M$$

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$$\Rightarrow \frac{E_{1}}{S} \int_{A_{1}} \nabla A + \frac{E_{2}}{S} \int_{A_{2}} \nabla A = M$$

$$\Rightarrow \frac{E_{1}}{S} \int_{A_{1}} \nabla A + \frac{E_{2}}{S} \int_{A_{2}} \nabla A = M$$

$$\Rightarrow \frac{E_{1}}{S} \int_{A_{1}} \nabla A + \frac{E_{2}}{S} \int_{A_{2}} \nabla A = M$$

$$\Rightarrow \frac{E_{1}}{S} \int_{A_{1}} \nabla A = M$$

$$\Rightarrow \frac{E_{$$

$$G_2 = -E_1 n \frac{y}{S}$$

$$dF_1 = \nabla_1 dA = -E_1 + dA$$

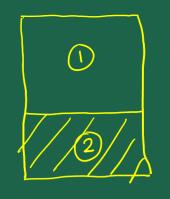
$$dF_2 = \nabla_2 dA = -E_1 + dA = -E_1 + dA$$

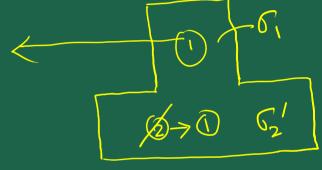
$$dF_2 = \nabla_2 dA = -E_1 + dA = -E_1 + dA$$

$$G_2 = \frac{dF_2}{dA} \quad G_2 \frac{dF_2}{ndA}$$

$$G_2 = nG_2$$

$$IIII$$





Transformed US

10. The reinforced concrete beam shown is subjected to a positive bending moment of 175 kN·m. Knowing that the modulus of elasticity if 25 GPa for the concrete and 200 GPa for the steel, determine the stress in the steel and the maximum stress in the concrete.

[212 MPa, −15.59 MPa]

